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		GENERAL NOTES:	
	1.	THE STRUCTURAL DRAWINGS MUST BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS AND THE SPECIFICATIONS. THE CONTRACTOR MUST VERIFY THE REQUIREMENTS OF OTHER TRADES FOR ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.	
C	2.	THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE, 2018 EDITION, AS ADOPTED BY THE UNIFIED FACILITIES CRITERIA, AND AS FOLLOWS:A.UFC 1-200-01DoD BUILDING CODE, OCTOBER 8, 2019 WITH CHANGE 1, OCTOBER 1, 2020B.UFC 3-220-01GEOTECHNICAL ENGINEERING NOVEMBER 1, 2012C.UFC 3-301-01STRUCTURAL ENGINEERING OCTOBER 1, 2019D.UFC 4-010-01DoD MINIMUM ANTITERRORISM STANDARDS FOR BUILDINGS DECEMBER 8, 2018 WITH CHANGE 1, AUGUST 1, 2020	
	3	THE WORK UNDER THE FOLLOWING SPECIFICATION SECTIONS IS SUBJECT TO	
	Э.	<ul> <li>SPECIAL INSPECTIONS AND TESTS AS DESCRIBED IN SECTION 313 3003LCT TO</li> <li>SPECIAL INSPECTIONS AND TESTS AS DESCRIBED IN SECTION 1704 OF THE</li> <li>INTERNATIONAL BUILDING CODE, 2018 EDITION:</li> <li>A. 033000 - CAST-IN-PLACE CONCRETE</li> <li>B. 034500 - PRECAST ARCHITECTURAL CONCRETE</li> <li>C. 051200 - STRUCTURAL STEEL</li> <li>D. 052100 - STEEL JOIST FRAMING</li> <li>E. 053000 - STEEL DECK</li> <li>F. 054000 - COLD-FORMED METAL FRAMING</li> <li>G. 312300.0020 - EXCAVATION AND FILL</li> <li>H. 316213.20 - PRECAST / PRESTRESSED CONCRETE PILES</li> </ul>	
C		THIS PROJECT IS SUBJECT TO IBC SPECIAL INSPECTION REQUIREMENTS AS MODIFIED BY UFC 3-301-01. THE STATEMENT AND SCHEDULE OF SPECIAL INSPECTIONS DEFINE THE SCOPE OF SPECIAL INSPECTIONS ARE INCLUDED WITH THE PROJECT SPECIFICATIONS. SPECIAL INSPECTIONS FOR WIND AND SEISMC RESISTANCE AND STRUCTURAL OBSERVATIONS FOR WIND REQUIREMENTS AND SEISMIC RESISTANCE ARE NOT REQUIRED SINCE THE NOMINAL WIND SPEED FOR THE PROJECT LOCATION IS LESS THAN 110 MPH AND THE RISK CATEGORY IS II. STRUCTURAL OBSERVATIONS FOR PROGRESSIVE COLLAPSE AVOIDANCE ARE NOT REQUIRED.	
	4.	THE CONTRACTOR MUST BE RESPONSIBLE FOR TEMPORARY SHORING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL PERMANENT SUPPORTS AND LATERAL BRACING ARE IN PLACE.	:
	5.	THE PROJECT SPECIFICATIONS ARE NOT SUPERSEDED BY THE STRUCTURAL NOTES BUT ARE INTENDED TO BE COMPLEMENTARY TO THEM. REFER TO THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS IN EACH SECTION	
	6.	SPECIFIC NOTES AND DETAILS ON THE DRAWINGS MUST TAKE PRECEDENCE OVER STRUCTURAL NOTES AND TYPICAL DETAILS.	
	7.	CONSULTANTS' DRAWINGS, INCLUDING STRUCTURAL DRAWINGS, ARE CONSIDERED SUPPLEMENTARY TO THE ARCHITECTURAL DRAWINGS. ANY OMISSIONS OR CONFLICTS, INCLUDING DIMENSIONS, BETWEEN VARIOUS ELEMENTS OF THE CONSULTANTS' DRAWINGS AND/OR SPECIFICATIONS MUST BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND CONTRACTNG OFFICER PRIOR TO PROCEEDING WITH THE WORK.	
3	8.	THE DOCUMENTS DEFINING THE STRUCTURE ARE INSTRUMENTS OF SERVICE PREPARED BY SPEIGHT, MARSHALL AND FRANCIS, P.C. FOR ONE USE ONLY. THE STRUCTURAL DOCUMENTS MUST NOT BE REPRODUCED, OR COPIED IN WHOLE OR IN PART BY THE CONTRACTOR OR SUBCONTRACTORS FOR PREPARATION OF SHOP DRAWINGS OR OTHER SUBMITTALS WITHOUT WRITTEN PERMISSION FROM THE ARCHITECT AND CONTRACTING OFFICER.	:
	9.	LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS: A. LIVE LOADS: 1. SLAB-ON-GRADE (UNIFORM) 250 PSF 2. ROOF 20 PSF	
		B. SNOW DESIGN DATA: 1. GROUND SNOW LOAD 5 PSF	
~		1.       ULTIMATE DESIGN WIND SPEED       139 MPH         2.       NOMINAL DESIGN WIND SPEED       108 MPH         3.       RISK CATEGORY       II         4.       WIND EXPOSURE       C         5.       INTERNAL PRESSURE COEFFICIENT       ±0.18         6.       WIND DIRECTIONALITY FACTOR, Kd       0.85         7.       GUST EFFECT FACTOR, Gf       0.85         8.       TOPOGRAPHIC FACTOR, Kzt       1.0         9.       GROUND ELEVATION FACTOR, Ke       1.0         10.       DESIGN WIND PRESSURES ARE DETERMINED AND APPLIED IN	(
4		D.       SEISMIC DESIGN DATA:         1.       RISK CATEGORY         2.       SEISMIC IMPORTANCE FACTOR         3.       SS         4.       S1         5.       SITE SPECIFIC SDS VALUES         5.       SITE SPECIFIC         6.       SDS (SITE SPECIFIC)         0.438g         7.       SD1 (SITE SPECIFIC)         8.       SEISMIC DESIGN CATEGORY	

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	GENERAL NOTES (CONTINUED):		CAST-IN-PLACE CONCRETE NOTES:	STEEL JO
	D. SEISMIC DESIGN DATA (CONTINUED): 9. DESIGN BASE SHEAR 99 KIPS (INCLUDING REDUNDANCY FACTOR)	1.	CAST-IN-PLACE CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE (ACI) "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14) AND COMMENTARY (ACI 318R-14)".	1. STEEL JO INSTITUTE
	10.       SEISMIC SHEAR ON PILES       554 KIPS         (INCLUDING REDUNDANCY FACTOR)       11.         11.       SEISMIC RESPONSE COEFFICIENT       0.009         12.       RESPONSE MODIFICATION FACTOR       5.0	2.	CONCRETE MUST BE NORMAL WEIGHT AND MUST OBTAIN 28 DAY COMPRESSIVE STRENGTHS AS FOLLOWS: A. EXTERIOR SLABS AND WALLS 5,000 PSI	2. PREPARE JOIST MEI MARK, NU
	<ol> <li>REDUNDANCY FACTOR</li> <li>1.3</li> <li>ANALYSIS PROCEDURE</li> <li>EQUIVALENT LATERAL FORCE</li> <li>BASIC SEISMIC-FORCE-RESISTING SYSTEM IS INTERMEDIATE REINFORCED PRECAST CONCRETE SHEAR WALLS</li> </ol>	3.	B.CONCRETE NOT OTHERWISE NOTED4,000 PSIREINFORCING MATERIALS MUST BE AS FOLLOWS:A.REINFORCING BARS - ASTM A615, GRADE 60, DEFORMED	3. THE JOIST BRIDGING FOR THE ADDITION
10.	E. RAIN DESIGN DATA: 1. 100 YEAR (1 HOUR RAINFALL) 4.25 INCHES DESIGN CRITERIA SERVICEABILITY LIMITS:	4.	ALL REINFORCING STEEL AND EMBEDDED ITEMS MUST BE ACCURATELY PLACED IN THE POSITIONS SHOWN AND ADEQUATELY TIED AND SUPPORTED BEFORE CONCRETE IS PLACED TO PREVENT DISPLACEMENT BEYOND PERMITTED TOLERANCES.	TO NET UI BE CLEAR REQUIREI UNLESS S OR END V
	A. ROOF DEFLECTIONS: 1. LIVE LOAD L/360 2. TOTAL LOAD L/240 B. LATERAL DRIFT H/480 C. LATERAL (OUT OF PLANE) L/240	5.	MINIMUM CONCRETE COVER FOR REINFORCING STEEL AS INDICATED ON THE DRAWINGS MUST GOVERN WHEN IN CONFLICT WITH ACI 318-14.	4. ROOF JOI 'COMPON
	D. WIND LOADS WERE TAKEN AS 0.42 TIMES THE WIND PRESSURE FOR DETERMINING DEFLECTION LIMITS		STRUCTURAL PRECAST CONCRETE NOTES:	USE A MA
11.	THE BUILDING HAS AN OCCUPANCY DESIGNATION OF LOW OCCUPANCY AND NO SPECIFIC THREATS TO THE BUILDING HAVE BEEN IDENTIFIED. THE BUILDING IS EXEMPT FROM THE REQUIREMENTS OF LIFE 4 010 01 ANTITERRORISM PROVISIONS	1.	STRUCTURAL PRECAST CONCRETE MUST BE DESIGNED IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE (ACI) "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318-14.	
12.	THE FIRST FLOOR IS A STRUCTURAL SLAB. ALL UTILITIES MUST BE SUPPORTED FROM THE SLAB AND UTILITIES MUST HAVE FLEXIBLE CONNECTIONS AS THE UTILITY	2.	SUBMIT SHOP DRAWINGS PREPARED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA FOR THE DESIGN OF STRUCTURAL PRECAST CONCRETE, INCLUDING DESIGN LOADINGS AND REACTIONS APPLIED TO THE	BOLTED T COORDIN
	ENTERS THE BUILDING. REFER TO OTHER DISCIPLINES DRAWINGS FOR SUPPORT DETAILS. AT LOCATIONS WHERE UTILTIES ENTER THE BUILDING, THE PENETRATION THROUGH THE FOUNDATION WALLS. PRECAST WALLS AND TURNDOWN SLAB EDGES	3	SUPPORTING STRUCTURE.	6. CONCENT REINFOR
	MUST BE 6" LARGER THAN THE UTILITY TO ALLOW 3" OF SPACE AROUND THE UTILITY.	0.	BE DEVELOPED BY THE PRECAST MANUFACTURER TO SUIT THE SPECIFIED LOADS AND CONNECTIONS MUST ACCOUNT FOR THERMAL MOVEMENT AND CREEP OF PRECAST MEMBERS. DETAIL ALL CONNECTIONS ON SHOP DRAWINGS.	<ol> <li>UNLESS C</li> <li>DESIGN J<sup>i</sup></li> </ol>
1.	FOUNDATION NOTES: FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL EXPLORATION REPORT PREPARED	4.	IN ADDITION TO THEIR OWN DEAD WEIGHT AND THE DEAD LOADS SHOWN, PRECAST MEMBERS MUST BE DESIGNED TO SUPPORT THE LIVE LOADS INDICATED IN THE GENERAL NOTES.	DESIGNAT THE END
2.	BY TERRACON CONSULTANTS, INC. DATED DECEMBER 14, 2021. PRIOR TO PLACING FOUNDATION CONCRETE, ALL FOUNDATION EXCAVATIONS	5.	CONCRETE MUST BE NORMAL WEIGHT AND MUST OBTAIN A 28 DAY COMPRESSIVE STRENGTH OF 5,000 PSI.	STEEL DE 1. STEEL DE
	MUST BE INSPECTED BY THE GEOTECHNICAL ENGINEER TO EXPLORE THE EXTENT OF LOOSE, SOFT OR OTHERWISE UNSATISFACTORY SOIL MATERIAL. REPORT UNSUITABLE CONDITIONS TO THE CONTRACTING OFFICER AND PERFORM CORRECTIVE ACTION AS DIRECTED BY THE CONTRACTING OFFICER	6.	REINFORCING MATERIALS MUST BE AS FOLLOWS: A. REINFORCING BARS: ASTM A615, GRADE 60, DEFORMED B. WEI DED WIRE FABRIC: ASTM A185	AND STEE STEEL ST MANUAL F
3.	NO UNBALANCED BACKFILLING MAY BE DONE AGAINST WALLS UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING, EITHER BY TEMPORARY	7.	PRESTRESSING TENDONS MUST COMPLY WITH ASTM A416 GRADE 270, UNCOATED, 7-WIRE STRESS-RELIEVED STRAND.	2. STEEL DE A. 11 SF
			STRUCTURAL STEEL NOTES:	
1.	PILING MUST COMPLY WITH PRECAST CONCRETE INSTITUTE MNL-116 AND	1.	STRUCTURAL STEEL HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOURTEENTH EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION	FA PL
	MUST BE 14 INCH SQUARE PRECAST, PRESTRESSED CONCRETE SECTIONS.		(AISC 360-16) "STEEL CONSTRUCTION MANUAL" - LOAD & RESISTANCE FACTOR DESIGN.	3. WELDING CODE - SH
2.	PILE FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL EXPLORATION REPORT PREPARED BY TERRACON CONSULTANTS, INC. DATED DECEMBER 14, 2021.	2.	A. STRUCTURAL STEEL MUST COMPLY WITH THE FOLLOWING SPECIFICATIONS: A. STRUCTURAL STEEL W- AND WT-SHAPES - ASTM A992, Fy=50 KSI B. RECTANGULAR AND SQUARE HSS - ASTM A500, GRADE C, Fy=50 KSI C. ANCHOR RODS - ASTM F1554, GRADE 36	4. DECK OPI SUPPORT
3.	CONCRETE MUST BE NORMAL WEIGHT AND MUST OBTAIN A 28 DAY COMPRESSIVE STRENGTH OF 7,000 PSI.		D. ALL OTHER STRUCTURAL STEEL SHAPES, PLATES AND BARS - ASTM A36, Fy=36 KSI (UNLESS OTHERWISE NOTED)	ADJACEN DETAILED LOCATION
4.	PILE ALLOWABLE (UNLESS OTHERWISE NOTED) DESIGN CAPACITIES ARE AS         FOLLOWS (SEE A3/SB501 'PILE DESIGN CRITERIA' FOR ADDITIONAL INFORMATION):         A.       COMPRESSION =         55 TONS         B.       TENSION =         20 TONS	3.	"TYPICAL BOLTED WEB PLATE CONNECTION DETAILS" ON SHEET SF501 USING ASTM A325 BOLTS.	5. PERMANE FIXTURES
	C. LATERAL: FREE HEAD CONNECTION 5 TONS D. ULTIMATE DRIVING RESISTANCE 233 TONS E. DRIVING RESISTANCE IS BASED ON FACTOR OF SAFETY OF 2.5 AND A	4.	HIGH STRENGTH BOLTS MAY BE TIGHTENED TO THE "SNUG TIGHT" CONDITION IN LIEU OF FULL PRETENSIONING.	ROOF DEC
5.	46 TONS ALLOWANCE FOR DOWNDRAG. BASE BID LENGTH ON DRIVING ALL PILES EXCEPT TEST PILES TO AN ACTUAL TIP ELEVATION OF - 36.5 EFET	5.	WELDING MUST BE IN ACCORDANCE WITH AWS D1.1, "STRUCTURAL WELDING CODE - STEEL". WELD ELECTRODES MUST BE E70XX. CONTINUOUS 3/16" FILLET WELDS ARE REQUIRED UNLESS OTHERWISE NOTED.	LOADS TO OF 200 PC MUST NO FOR CON
6.	DRIVE 6 - 65 FOOT LONG TEST PILES IN LOCATIONS SHOWN ON THE FOUNDATION PLANS.	6.	SEE THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR ADDITIONAL STEEL (IF ANY) NOT SHOWN ON THE STRUCTURAL DRAWINGS.	THE RESU EXCEED 5
7.	TEST PILES SHOWN ON PLANS MUST BE TESTED USING PILE DRIVING ANALYZER (PDA) TO MEASURE DRIVING STRESSES, EVALUATE HAMMER PERFORMANCE AND VERIFY PILE CAPACITIES. TESTS MUST BE DONE IN ACCORDANCE WITH THE SPECIFICATIONS AND UNDER THE DIRECTION OF THE GEOTECHNICAL ENGINEER.	7.	UNLESS OTHERWISE NOTED, THE TOP OF ALL STEEL COLUMNS MUST HAVE A STEEL CAP PLATE. UNLESS OTHERWISE DETAILED OR NOTED, MINIMUM CAP PLATE DIMENSIONS MUST MATCH COLUMN WIDTH AND DEPTH AND MINIMUM THICKNESS OF CAP PLATE MUST EQUAL COLUMN WEB THICKNESS (1/2" MIN).	<ol> <li>PREPARE TYPES OF CLOSURE</li> <li>TOUCH-UI</li> </ol>
8.	PROVIDE AN AS-INSTALLED PILE DRAWING FOR EACH PILE WITH PILE LOCATION DISCREPANCIES NOTED ON THE DRAWING.	8.	STRUCTURAL STEEL NOTED TO BE GALVANIZED MUST CONFORM TO ASTM A123 OR ASTM A153. GALVANIZE STRUCTURAL STEEL AFTER FABRICATION WHERE PRACTICAL. REPAIR DAMAGED GALVANIZED COATING USING ASTM A780 ZINC-	RICH PAIN
9.	PILING MUST BE DRIVEN TO A MAXIMUM TOLERANCE IN ANY DIRECTION OF THREE INCHES PER PILE. WHERE AN INDIVIDUAL PILE IS DRIVEN OUT OF POSITION MORE THAN THREE INCHES IN ANY DIRECTION AND/OR WHERE THE CENTER OF GRAVITY OF A PILE GROUP IS OUT OF POSITION MORE THAN TWO INCHES, THE CONTRACTOR MAY BE REQUIRED TO DRIVE AN ADDITONAL PILE OR PILES TO COMPENSATE FOR THE ECCENTRICITY OF THE PILE AND/OR PILE GROUP	9.	RICH PAINT. SPLICE CONTINUOUS ANGLES AND PLATES AT SUPPORTS ONLY. SEE 'TYPICAL CONTINUOUS BENT PLATE OR ANGLE SPLICE DETAIL' ON SHEET SF502.	SPEIGHT MARSHALL FRAN STRUC TURAL ENGINEE 1228 PERIMETER PARKWAY, SUITE
				VIRGINIA BEACH, VIRGINIA 234 P: 757.427.1020 7814 CAROUSEL LANE SUITE 2 RICHMOND, VIRGINIA 23294 P: 804.464.4111

## DIST NOTES:

DISTS MUST CONFORM TO THE LATEST EDITION OF THE STEEL JOIST E (SJI) STANDARD SPECIFICATIONS.

E AND SUBMIT FOR REVIEW SHOP DRAWINGS SHOWING THE LAYOUT OF EMBERS, SPECIAL CONNECTIONS, JOINING AND ACCESSORIES. INCLUDE UMBER, TYPE, LOCATION AND SPACING OF JOISTS AND BRIDGING.

T MANUFACTURER MUST BE RESPONSIBLE FOR CONTINUOUS JOIST G LINES SATISFYING THE REQUIREMENTS OF THE SJI SPECIFICATION TOP AND BOTTOM CHORDS OF ALL STEEL JOISTS, AS WELL AS ANY VAL BRIDGING/BRACING SHOWN OR REQUIRED FOR JOISTS SUBJECTED JPLIFT OR OTHER SPECIAL LOADS. ALL REQUIRED JOIST BRIDGING MUST RLY INDICATED ON THE SHOP DRAWINGS. CROSS-BRIDGING IS ED AT CHANGES OF JOIST DEPTHS AND ENDS OF ALL BRIDGING LINES, SUCH ENDS ARE PROPERLY ANCHORED INTO INTERSECTING INTERIOR WALLS. ALL BRIDGING AND BRIDGING ANCHORS MUST BE COMPLETELY ED BEFORE PLACING OF ANY CONSTRUCTION LOADS ON THE JOISTS.

ISTS MUST BE DESIGNED FOR UPLIFT LOADS INDICATED IN THE NENT AND CLADDING DESIGN PRESSURES' AND 'COMPONENT AND IG LEGEND' SHOWN ON SHEET S-002 TO OBTAIN THE NET UPLIFT VALUE. AXIMUM DEAD LOAD OF 5 PSF.

RD STRESS INCREASE NOT PERMITTED FOR LOAD COMBINATIONS NG WIND PER IBC SECTION 1605.3.1.1.

IOIST SPAN EQUALS 40 FEET OR GREATER JOIST MUST BE ERECTION TO STEEL BEAMS, UNLESS PANELIZED JOIST ERECTION IS EMPLOYED. NATE BOLT HOLE LOCATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS.

TRATED LOADS NOT LOCATED AT JOIST PANEL POINTS MUST BE RCED PER 'TYPICAL DETAIL AT CONCENTRATED LOADS ON JOISTS'.

OTHERWISE NOTED, JOIST SEAT DEPTHS MUST BE 6 1/2 ".

JOIST EXTENSIONS FOR THE SAME UNIFORM LOAD AS THE JOIST TION AND FOR AN ADDITIONAL CONCENTRATED LOAD OF 200 LBS AT OF THE EXTENSION.

### ECK NOTES:

ECK MUST CONFORM TO THE LATEST EDITION OF THE AMERICAN IRON EL INSTITUTE (AISI), "SPECIFICATION FOR THE DESIGN OF COLD-FORMED TRUCTURAL MEMBERS" AND THE STEEL DECK INSTITUTE (SDI), "DESIGN FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS."

ECK UNITS MUST COMPLY WITH THE FOLLOWING:

1/2" ROOF DECK MUST BE 18 GAGE GALVANIZED TYPE B RIB DECK PANNING PERPENDICULAR TO SUPPORTS UNLESS OTHERWISE NOTED. TTACH DECK TO STRUCTURAL STEEL SUPPORTS WITH 5/8" DIAMETER UDDLE WELDS AT EVERY RIB, INCLUDING RIBS WHERE SIDELAPS OCCUR. ASTEN ADJACENT DECK UNITS WITH #10 SELF-TAPPING HEX HEAD CREWS AT 1/3 POINTS BETWEEN SUPPORTS ALONG SIDELAPS AND ASTEN EDGEMOST DECK PANEL TO STEEL FRAMING WITH 5/8" DIAMETER UDDLE WELDS AT SAME SPACING AS SIDELAP FASTENERS.

G MUST BE IN ACCORDANCE WITH AWS D1.3 "STRUCTURAL WELDING HEET STEEL". FIELD WELDS MUST BE WIRE BRUSHED AND PAINTED IC RICH PAINT.

PENINGS MEASURING GREATER THAN 12" ON ANY SIDE MUST HAVE TS ON ALL SIDES OF OPENING. SUPPORTS MUST SPAN BETWEEN IT BEAMS OR JOISTS ON TWO SIDES. UNLESS OTHERWISE NOTED OR D ON THE DRAWINGS, USE 4x4x1/4" ANGLES COORDINATE OPENING SIZES, INS AND DETAILS WITH ARCHITECTURAL, MECHANICAL, PLUMBING AND CAL DRAWINGS.

ENT SUSPENDED LOADS SUCH AS SUSPENDED CEILINGS, LIGHT , DUCTS, OR OTHER UTILITIES MUST NOT BE SUPPORTED BY STEEL ECK.

STEEL DECK ERECTION AND ROOFING, DISTRIBUTE CONSTRUCTION O PREVENT DAMAGE TO DECK. CONCENTRATED CONSTRUCTION LOADS OUNDS OR LESS DISTRIBUTED OVER A 1'-0" WIDE SECTION OF DECK OT REQUIRE ANY FURTHER DISTRIBUTION. USE WORKING PLATFORMS ICENTRATED CONSTRUCTION LOADS OF OVER 200 POUNDS, SUCH THAT SULTING UNIFORM CONSTRUCTION LOAD ON THE DECK DOES NOT 50 PSF.

E AND SUBMIT SHOP DRAWINGS FOR REVIEW SHOWING THE LAYOUT AND F DECK UNITS, ANCHORAGE DETAILS AND CONDITIONS REQUIRING E STRIPS, SUPPLEMENTARY FRAMING AND OTHER ACCESSORIES.

JP ALL SCARRED, ABRADED AND RUST AREAS IN STEEL DECK WITH ZINC NT.



NOTE: IF THIS DRAWING IS LESS THAN 34" X 22" IT IS A REDUCED SIZE DRAWING



		COLD-FORMED METAL FRAMING NOTES:
	1.	COLD-FORMED METAL FRAMING MUST BE DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) "COLD-FORMED STEEL DESIGN MANUAL". THE CONTRACTOR MUST SUBMIT SHOP DRAWINGS PREPARED BY A PROFESSIONAL ENGINEER REGISTERED FOR THE DESIGN OF COLD-FORMED METAL FRAMING, INCLUDING DESIGN LOADINGS AND REACTIONS APPLIED TO THE SUPPORTING STRUCTURE.
D	2.	COLD-FORMED METAL FRAMING MEMBERS MUST BE FORMED OF CORROSION- RESISTANT STEEL CONFORMING TO ASTM A653 WITH A MINIMUM YIELD STRENGTH OF 33 KSI FOR TRACKS AND RUNNERS AND (50 KSI FOR 54 MILS AND HEAVIER).
	3.	WELDING MUST BE IN ACCORDANCE WITH AWS D1.3, "STRUCTURAL WELDING CODE - SHEET STEEL".
	4.	COLD-FORMED METAL FRAMING MEMBERS MUST BE DESIGNED FOR THE LOADS GIVEN IN THE GENERAL NOTES. UNLESS OTHERWISE NOTED, THE DESIGN LOADS ARE UNFACTORED SERVICE LOADS; ALL APPLICABLE FACTORS MUST BE APPLIED.
	5.	MAXIMUM LATERAL DEFLECTION OF COLD-FORMED FRAMING MEMBERS MUST BE L/360.
	6.	COLD-FORMED METAL FRAMING MEMBERS MUST BE DESIGNED FOR ALL FRAMING CONDITIONS FOUND IN THE STRUCTURE, INCLUDING WALLS, CORNERS, HEADERS, JAMBS, EXTERIOR SOFFITS AND GUTTER SUPPORTS. SOME CONDITIONS MAY REQUIRE MODIFICATION OF COLD-FORMED FRAMING MEMBERS (SUCH AS NOTCHING OR REVISING SIZES) OR MULTIPLE STUDS TO SUPPORT INCREASED LOADS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE ALL CONDITIONS AND DETAILS.
С	7.	COLD-FORMED METAL FRAMING CONNECTIONS TO THE STRUCTURE MUST BE DESIGNED FOR ALL FRAMING CONDITIONS. CONNECTIONS ARE SCHEMATICALLY SHOWN ON THE STRUCTURAL DRAWINGS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE ALL CONNECTION CONDITIONS AND LOCATIONS.
U		POST-INSTALLED ANCHOR NOTES:
	1.	POST-INSTALLED ANCHORS MUST BE CAPABLE OF WITHSTANDING DESIGN LOADS INDICATED ON THE DRAWINGS. APPLICABLE POST-INSTALLED ANCHOR TYPE(S) ARE AS FOLLOWS: A. ANCHORAGE TO CRACKED AND/OR UNCRACKED CONCRETE: 1 ADHESIVE ANCHORS OR MECHANICAL ANCHORS, AS INDICATED
	2.	ANCHOR CAPACITY MUST BE BASED ON TECHNICAL DATA PUBLISHED BY THE ANCHOR MANUFACTURER OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. CONTRACTOR MUST PROVIDE CALCULATIONS DEMONSTRATING THE CHOSEN ANCHOR IS CAPABLE OF
		ACHIEVING THE PERFORMANCE VALUES INDICATED. PRODUCTS SUBMITTED WILL BE EVALUATED BY SHOWING ICC ESR COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.
	3.	INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS INCLUDED IN ANCHOR PACKAGING.
В	4.	THE CONTRACTOR MUST ARRANGE A REPRESENTATIVE OF THE ANCHOR MANUFACTURER TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL ANCHORING PRODUCTS. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
	5.	ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
	6.	EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS OTHERWISE NOTED, THE REINFORCING BARS CANNOT BE CUT. THE CONTRACTOR MUST REVIEW THE EXISTING STRUCTURAL DRAWINGS AND MUST LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS BY GPR X-RAY, CHIPPING OR OTHER MEANS.
A		
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	L	

### DELEGATED DESIGN NOTES:

DESIGN RESPONSIBILITY FOR THE FOLLOWING ENGINEERED SYSTEMS AND COMPONENTS IS DELEGATED TO A QUALIFIED SPECIALTY STRUCTURAL ENGINEER RETAINED BY THE CONTRACTOR. THESE SYSTEMS AND COMPONENTS INCLUDE, BUT ARE NOT LIMITED TO:

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OPEN-WEB STEEL JOISTS Α

3.

- PRECAST CONCRETE PILES
- PRECAST CONCRETE WALL PANELS С
- COLD FORMED METAL FRAMING
- DELEGATED ENGINEERED SYSTEMS AND COMPONENTS MUST SATISFY ASCE 7 2. AND REQUIREMENTS OF APPLICABLE MATERIAL-SPECIFIC STANDARDS.
- COORDINATE WITH THE CONTRACT DOCUMENTS FOR PROFESSIONAL LICENSURE AND SEALING REQUIREMENTS, DESIGN CRITERIA, DETAILS OF THE SYSTEM/COMPONENT INTERFACE WITH THE PRIMARY STRUCTURE AND SUBMITTAL AND CALCULATION REQUIREMENTS.
- DO NOT FABRICATE OR INSTALL DELEGATED DESIGN ITEMS UNTIL SUBMITTED 4 DELEGATED DESIGN DOCUMENTS HAVE BEEN REVIEWED FOR COMPLIANCE BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.
- SEE THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL 5. DRAWINGS AND THE SPECIFICATIONS FOR ADDITIONAL DELEGATED DESIGN REQUIREMENTS (IF ANY) NOT SHOWN ON THE STRUCTURAL DRAWINGS.

COMPONENTS AND CLADDIN								
ROOF	TRIBU							
ZONE	1	0						
1	+18.9	-50.3	+16					
2	+18.9	-58.2	+16					
3	+18.9	-77.8	+16					
WALL			TRIBU					
ZONE	1							
4	+42.4	-46.0	+38					
5	+42.4	-56.6	+38					
COMPONE	COMPONENTS AND CLADDING DESIGN PRES							
1. PLUS AND MINUS SIGNS SIGNIFY PRESS								
2. WID	TH OF COEFFIC	CIES, RESPECTI CIENT PRESSUR	VELY. RE ZONE					

SHEET FOR LOCATIONS. CLADDING LEGEND' ON THIS SHEET FOR LOCATIONS. ALL WIND PRESSURES ARE ULTIMATE LOADS. FOR SERVICE LOAD PRESSURES, MULTIPLY THE ABOVE VALUES BY 0.6 PER ASCE 7-16.

COMPONENT AND CLADDING LEGEND NOT TO SCALE

2

3

# CAST-IN-PLACE CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE

	TENSION BARS											COMPRESSION BARS					
AR	f'c = 3,000 PSI				f'c = 4,000 PSI				f'c = 5,000 PSI			f'c = 6,000 PSI				f'c = ALL	
ZE	REG	REGULAR		OP	REGI	JLAR	т	OP	REG	REGULAR TOP		REGULAR		TOP			
y = KSI	CLA	ASS	CLA	ASS	CLA	<b>NSS</b>	CLA	ASS	CLA	ASS	CL/	ASS	CLA	ASS	CLA	SS	
	А	В	Α	В	Α	В	A	В	Α	В	Α	В	Α	В	Α	В	
<b>#</b> 3	17"	22"	22"	28"	15"	19"	19"	25"	13"	17"	17"	22"	12"	16"	16"	20"	12"
<b>#</b> 4	22"	29"	29"	38"	19"	25"	25"	33"	17"	23"	23"	29"	16"	21"	21"	27"	15"
<b>#</b> 5	28"	36"	36"	47"	24"	31"	31"	41"	22"	28"	28"	36"	20"	26"	26"	33"	19"
<b>#</b> 6	33"	43"	43"	56"	29"	37"	37"	49"	26"	34"	34"	44"	24"	31"	31"	40"	23"
<b>‡</b> 7	48"	63"	63"	81"	42"	54"	54"	71"	38"	49"	49"	63"	34"	45"	45"	58"	27"
<b>#</b> 8	55"	72"	72"	93"	48"	62"	62"	81"	43"	56"	56"	72"	39"	51"	51"	66"	30"
<b>#</b> 9	62"	81"	81"	105"	54"	70"	70"	91"	48"	63"	63"	81"	44"	57"	57"	74"	34"
10	70"	91"	91"	118"	61"	79"	79"	102"	54"	71"	71"	92"	50"	64"	64"	84"	39"
11	78"	101"	101"	131"	67"	87"	87"	114"	60"	78"	78"	102"	55"	71"	71"	93"	43"

NOTES (THESE NOTES SHALL BE USED FOR ALL SPLICES UNLESS NOTED OTHERWISE ON DRAWINGS):

1. TOP BARS ARE HORIZONTAL BARS, SPLICED SO THAT 12" OR MORE OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCING BAR.

2. CLASS A SPLICES MAY BE USED ONLY WHEN 50% OR LESS OF THE BARS ARE SPLICED WITHIN THE LAP SPLICE LENGTH.

3. CLASS B SPLICES SHALL BE USED FOR ALL SPLICES IN SLABS, BEAMS, JOISTS, WALLS, MOMENT RESISTING COLUMNS, AND JAMB COLUMNS, UNLESS THEY MEET THE REQUIREMENTS OF NOTE #2 ABOVE.

4. TIES AND STIRRUPS SHALL NOT BE SPLICED UNLESS APPROVED BY THE ENGINEER OF RECORD. ROUND COLUMN TIES MAY BE SPLICED USING CLASS A LAPS.

5. FOR ALL LIGHTWEIGHT CONCRETE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.

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6. THE BAR LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.5 WHEN EITHER OF THE FOLLOWING IS TRUE: A. CLEAR SPACING OF BARS BEING DEVELOPED IS LESS THAN ONE BAR DIAMETER, CLEAR COVER IS LESS THAN ONE BAR DIAMETER AND STIRRUPS OR TIES ALONG THE LENGTH OF THE SPLICE ARE LESS THAN THE CODE MINIMUM.

B. CLEAR SPACING OF BARS BEING DEVELOPED IS LESS THAN 2 BAR DIAMETERS AND CLEAR COVER IS LESS THAN ONE BAR DIAMETER.



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NOTE: IF THIS DRAWING IS LESS THAN 34" X 22"
IT IS A REDUCED SIZE DRAWING

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STRUCTURAL ABBREVIATION LIST:								
±	PLUS/MINUS	K.S.F.	KIPS PER SQUARE FUUT					
Ø		K.S.I.	KIPS PER SQUARE INCH					
CL	CENTERLINE							
0/C	ONCENTER	LLH	LONG LEG HORIZONTAL					
		LLV	LONG LEG VERTICAL					
A.B.	ANCHOR BOLT	LSH	LONG SIDE HORIZONTAL					
ACI	AMERICAN CONCRETE INSTITUTE	LSV	LONG SIDE VERTICAL					
AISC	AMERICAN INSTITUTE OF							
	STEEL CONSTRUCTION	M.O.S.	MIDDLE OF SLAB					
A.R.	ANCHOR ROD	M.O.W.	MIDDLE OF WALL					
ASTM	AMERICAN SOCIETY FOR	MANUF.	MANUFACTURER or					
	TESTING AND MATERIALS		MANUFACTURER'S					
ADJ.	ADJACENT	MATL.	MATERIAL					
ARCH.	ARCHITECT or ARCHITECTURAL	MAX.	MAXIMUM					
		MECH	MECHANICAI					
В	BOTTOM	MIN	MINIMUM					
B.D.								
BLDC		NTS						
		N.T.O.	NOT TO SCALE					
			ODDOSITE					
		UPP.	OPPOSITE					
BRG.	BEARING							
BIWN.	BEIWEEN	PAF	POWDER ACTUATED FASTENER					
		P.C.	PRECASI					
C.G.	CENTER OF GRAVITY	PLF	POUNDS PER LINEAR FOOT					
C.I.P.	CAST IN PLACE	PSF	POUNDS PER SQUARE FOOT					
CANT.	CANTILEVER	PSI	POUNDS PER SQUARE INCH					
COL.	COLUMN	PLUMB.	PLUMBING					
CONC.	CONCRETE	PROJ.	PROJECTION					
CONN.	CONNECT or CONNECTION							
CONT.	CONTINUOUS	R.	RADIUS					
COORD.	COORDINATE	REF.	REFERENCE					
		REINF.	REINFORCED or REINFORCING					
DBL.	DOUBLE	REQD.	REQUIRED					
DET.	DETAIL	REV.	REVISION					
DIA.	DIAMETER		-					
DIAG.	DIAGONAL	SLO	SHORT LEG OUTSTANDING					
DWG	DRAWING	SDI	STEEL DECK INSTITUTE					
2		SE.I	SEISMIC EXPANSION JOINT					
FF	FACH FACE	S.11						
E.I.	EDGE OF	50G	SI AB-ON-GRADE					
E.U.		S.F.						
			SCHEDULE					
	ELECTRICAL		SECTION					
		3.I.N.D.A.						
		OTO						
EQUIV.	EQUIVALENT							
EXP.	EXPANSION	STIR.	STIRRUP					
		STRUCT.	STRUCTURAL					
F.L.	FULLLENGIH	SQ.	SQUARE					
F.U.		<b>T</b> 0 -						
F.R.	FIRST RISER	T.O.S.	TOP OF STEEL					
FIN.	FINISH or FINISHED	IEMP.	TEMPERATURE					
FTG.	FOOTING	TYP.	TYPICAL					
~ ~								
G.C.	GENERAL CONTRACTOR	U.O.N.	UNLESS OTHERWISE NOTED					
GALV.	GALVANIZED							
		VEKI.	VERTICAL					
HORIZ.	HURIZUNTAL							
		۷۷.۲.						
J.B.E.	JUIST BEAKING ELEVATION	VV.VV.⊢.						





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ALL (-X)	SEISMIC LOAD SHEAR AT TOP OF WALL (Vex, KIPS)	WIND LOAD SHEAR AT TOP OF WALL (Vwx, KIPS)
	55.0	45.0
	27.0	25.0
	27.0	23.0
	7.0	9.0
	22.0	19.0
	37.0	29.0
	9.0	6.0
	13.0	11.0







# **GENERAL FOUNDATION PLAN NOTES:**

- A. SEE THE CIVIL DRAWINGS FOR ACTUAL FINISHED FLOOR ELEVATION. TOP OF SLAB EQUALS FINISHED FLOOR ELEVATION AND MUST SERVE AS THE **REFERENCE ELEVATION 0'-0".**
- B. FOR THE STRUCTURAL NOTES AND ABBREVIATIONS SEE SHEETS S-001 AND S-002.
- SEE THE DIMENSION PLAN ON SHEET SB101 AND THE ARCHITECTURAL C. DRAWINGS FOR DIMENSIONS NOT SHOWN. SEE THE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY CONTROL JOINTS AND PRECAST CONCRETE DETAILS
- D. TOP OF ALL PILE CAPS AND GRADE BEAMS MUST BE 2'-0" BELOW REFERENCE ELEVATION 0'-0", UNLESS OTHERWISE NOTED.
- E. PILE CAP AND GRADE BEAM CENTERLINES MUST COINCIDE WITH COLUMN CENTERLINES, UNLESS OTHERWISE NOTED.

# **X** FOUNDATION PLAN KEY NOTES :

- 1. FIRST FLOOR AT MUST BE A 12" THICK STRUCTURAL SLAB REINFORCED WITH #6 AT 6" ON-CENTER MAXIMUM, TOP AND BOTTOM (OUTER LAYER) AND #5 AT 12" ON-CENTER, TOP AND BOTTOM (INNER LAYER) OVER 15 MIL. VAPOR RETARDER (MINIMUM), OVER 4" DEPTH OF POUROUS FULL.
- 2. FACE OF 8" CONCRETE FOUNDATION WALL BELOW.
- 3. NOTCH PRECAST TO SPAN OVER STEEL COLUMN BASE PLATE.
- 4. WALKWAY MUST BE AN 8" THICK STRUCTURAL SLAB REINFORCED WITH #4 AT 12" ON-CENTER MAXIMUM EACH WAY.
- 5. SEE DETAIL B4/SB501 FOR PRECAST WALL TO PILE CAP UPLIFT CONNECTION.
- 6. TURN DOWN SLAB ONTO GRADE BEAM PER SECTION C1/SB401.
- 7. TURN DOWN SLAB ONTO GRADE BEAM PER SECTION C2/SB402.









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2' 3' 4'

5'

0 4" 1' 2' 3/4" = 1'-0" GRAPHIC SCALE

NOTE: IF THIS DRAWING IS LESS THAN 34" X 22" IT IS A REDUCED SIZE DRAWING



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# **TYPICAL COLUMN ANCHOR ROD DETAIL**

THREADED ANCHOR

**BASE PLATE DETAILS** 

PROJECTION AS

REQUIRED (TYP.)

SHOWN) - FOR

(REINFORCING NOT

**ROD - SEE COLUMN** 

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# **TYPICAL COLUMN ISOLATION JOINT DETAIL**





COLUMN BASE



(C4)

1 1/2"

- 4 4 4 4 4 4 PIPE SLEEVES. WHERE SLEEVES CANNOT BE PLACED IN ACCORDANCE WITH THIS DETAIL (OR SHIFTED TO SUIT) IMMEDIATELY CONTACT THE ARCHITECT. NO PIPE SLEEVES ARE ALLOWED IN PILE CAPS OR WITHIN
- MECH. AND ELEC. DWGS. 2" CLR. MIN. - TOP REINF. STIRRUP 4" CLR. MIN. BOTT. REINF 4 A A A A NOTE: UNDER NO CIRCUMSTANCES MUST ANY TOP OR BOTTOM REINFORCING OR STIRRUPS BE DISCONTINUED TO PLACE

PIPE SLEEVE THRU GRADE BEAM

-PIPE - FOR EXACT SIZE INVERT ELEVATION AND SLEEVE FOR PIPE -DIAMETER = PIPE LOCATION COORDINATE DIA. + 2" (TYP. U.O.N.) \_\_\_\_ WITH THE CIVIL, PLUMB.

# SECTION THRU GRADE BEAM INTERIOR CLEAR SPAN = L1

—2" CLR. TO

AT TOP

(TYP.) –

—3" CLR. TO

STIRRUPS

AT BOTT. ----- SCHEDULE -

. 4 4 . 4

ab ab an |--- --- de ---- ---- ----

STIRRUPS

AND SIDES

"W" WIDTH

ADDED SIDE BARS

(IF ANY) - SEE

# TYPICAL CONCRETE GRADE BEAM DETAIL NOTES:

- WHERE AREA OF TOP BARS IN ADJACENT SPANS DIFFER, CARRY COMBINATION WITH GREATEST AREA TO MIDSPAN OF ADJACENT SPAN.
- DISCONTINUE 50% MAXIMUM OF TOP BARS WITHIN MIDDLE THIRD OF SPAN TO DIMENSIONS INDICATED. OUTSIDE FACE BARS ARE ALWAYS FULL LENGTH. ALL TOP BARS INDICATED SHALL BE PRESENT AT CANTILEVERS AND OVER SUPPORTS
- BOTTOM BARS ALTERNATE 50% MINIMUM 'LONG' BARS AND 50% MAXIMUM 'SHORT' BARS; OUTSIDE FACE BARS ARE 'LONG' BARS.
- PROVIDE #5 AT 12" ON-CENTER ADDED SIDE BARS EACH SIDE OF GRADE BEAM.

CONCRETE GRADE BEAM SCHEDULE										
IARK	"W" עדסועע	"D"	TOP BARS		BOTTOM BARS		STIRRUPS			
	(INCH)	(INCH) (INC	(INCH) (INC	(INCH)	QUAN.	SIZE	QUAN.	SIZE	SIZE	SPA.
B-1M	24	24	4	#11	4	#11	#5	8"o/c		
GB-2	36	30	6	#8	6	#8	#4	12"o/c	NOTE 4	
GB-3	24	30	4	#8	4	#8	#4	6"o/c	NOTE 4	
GB-4	30	48	6	#8	6	#8	#4	12"o/c	NOTE 4	
GB-5	48	34	8	#8	8	#8	#4	8"o/c	NOTE 4	

1 1/2"

1 5/16"Ø HOLES

FOR 3/4"Ø A.R.

COLUMN -

SEE PLAN



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DRAWFORM REVISION: 25 AUG 2020



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1	NOTE
	F THIS DRAWING IS LESS THAN 34" X 22"
	IT IS A REDUCED SIZE DRAWING
	IT IS A REDUCED SIZE DRAWING





# GENERAL FRAMING PLAN NOTES :

- A. SEE THE DIMENSION PLAN ON SHEET SB101 AND THE ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN. SEE THE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY CONTROL JOINTS AND PRECAST CONCRETE DETAILS
- B. FOR THE STRUCTURAL NOTES AND ABBREVIATIONS SEE SHEETS SB001 AND SB002.

# X FRAMING PLAN KEY NOTES :

- . ROOF JOISTS AND OTHER ROOF DECK SUPPORTS MUST BE SPACED EQUALLY BETWEEN GIVEN LOCATIONS, WITH A MAXIMUM SPACING OF 6'-0" ON-CENTER, UNLESS OTHERWISE NOTED.
- 2. HSS2 1/2 x 2 1/x2 x1/4 OUTLOOKERS LOCATED AT EACH COLUMN AND AT 4'-0" ON-CENTER MAXIMUM SPACING.
- 3. HSS2 1/2 x 2 1/x2 x1/4 SLOPED OUTLOOKER.
- 4. HSS8x6x3/8 (LSV; B.O.S.=+12'-3 1/2", LOW) AND HSS8x6x3/8 (LSV; B.O.S.= +19'-3", HI).
- DOUBLE L4x4x1/4 AT EACH CEILING FAN. WELD EACH ANGLE TO JOIST BOTTOM CHORD WITH 2" OF 3/16" WELD. SEE THE ARCHITECTURAL DRAWINGS FOR LOCATION.
- 6. CONNECT PRECAST PANEL TO COLUMN AS REQUIRED TO BRACE THE PANEL OUT OF PLANE. DESIGN OF CONNECTION MUST BE BY PRECAST MANUFACTURER.
- 7. EXTEND DECK EDGE BENT PLATES ON THE TWO SIDES OF THE ROOF EDGE TO THE CORNER. BENT PLATES MUST BE CONTINUOUS FOR TWO SUPPORTS FROM THE CORNER. MITRE AND CONNECT THE VERTICAL AND HORIZONTAL LEGS OF THE PLATES AT THE CORNER WITH A BENT PLATE 1/4x6x2. REFER TO A2/SF502 FOR ADDITIONAL DETAILS AND WELD REQUIREMENTS
- 8. EXTEND COLUMN TO BOTTOM OF DECK ELEVATION AND WELD HSS2 1/2x2 1/2x1/4 SLOPE OUTLOOKER TO ALL AROUND TO OUTSIDE FACE OF COLUMN TO SUPPORT DECK EDGE BENT PLATE.
- 9. DESIGN CONNECTION BETWEEN PRECAST PANEL AND STEEL ROOF BEAM FOR OUT OF PLANE LOAD REACTION AT THE TOP OF THE PRECAST PANEL. THESE CONNECTIONS ARE IN ADDITIONAL TO CONNECTIONS INDICATED IN DETAIL C2/SF502.





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**BEAM OR GIRT** PARALLEL TO COLUMN FLANGE



**TYPICAL HSS TO COLUMN BOLTED CONNECTION DETAILS** NOT TO SCALE



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3/4" = 1'-0"

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HEET 56 OF 129

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CALE:

PROJECT NO .:

ONSTR. CONTR. NO.

IAVFAC DRAWING NO.